

PROBLEM 6.1

Three full-size 50 × 100-mm boards are nailed together to form a beam that is subjected to a vertical shear of 1500 N. Knowing that the allowable shearing force in each nail is 400 N, determine the largest longitudinal spacing s that can be used between each pair of nails.

SOLUTION

$$I = \frac{1}{12}bh^3 = \frac{1}{12}(100)(150)^3 = 28.125 \times 10^6 \text{ mm}^4$$

$$= 28.125 \times 10^{-6} \text{ m}^4$$

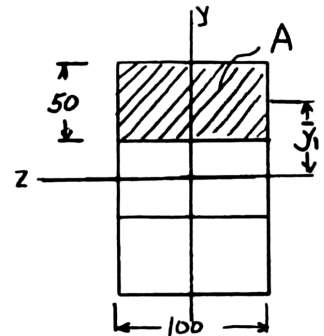
$$A = (100)(50) = 5000 \text{ mm}^2$$

$$\bar{y}_1 = 50 \text{ mm}$$

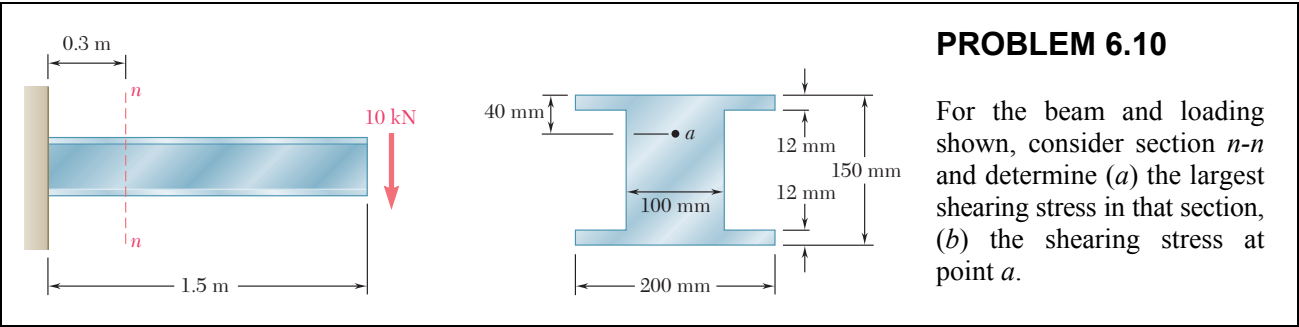
$$Q = A\bar{y}_1 = 250 \times 10^3 \text{ mm}^3 = 250 \times 10^{-6} \text{ m}^3$$

$$q = \frac{VQ}{I} = \frac{(1500)(250 \times 10^{-6})}{28.125 \times 10^{-6}} = 13.3333 \times 10^3 \text{ N/m}$$

$$qs = 2F_{\text{nail}} \quad s = \frac{2F_{\text{nail}}}{q} = \frac{(2)(400)}{13.3333 \times 10^3} = 60.0 \times 10^{-3} \text{ m}$$



$$s = 60.0 \text{ mm} \blacktriangleleft$$



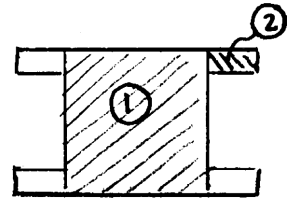
PROBLEM 6.10

For the beam and loading shown, consider section $n-n$ and determine (a) the largest shearing stress in that section, (b) the shearing stress at point a .

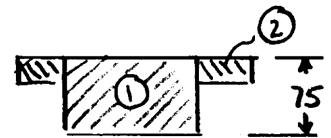
SOLUTION

At section $n-n$, $V = 10 \text{ kN}$.

$$\begin{aligned}
 I &= I_1 + 4I_2 \\
 &= \frac{1}{12}b_1h_1^3 + 4\left(\frac{1}{12}b_2h_2^3 + A_2d_2^2\right) \\
 &= \frac{1}{12}(100)(150)^3 + 4\left[\left(\frac{1}{12}\right)(50)(12)^3 + (50)(12)(69)^2\right] \\
 &= 28.125 \times 10^6 + 4\left[0.0072 \times 10^6 + 2.8566 \times 10^6\right] \\
 &= 39.58 \times 10^6 \text{ mm}^4 = 39.58 \times 10^{-6} \text{ m}^4
 \end{aligned}$$



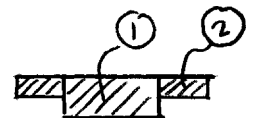
$$\begin{aligned}
 (a) \quad Q &= A_1\bar{y}_1 + 2A_2\bar{y}_2 \\
 &= (100)(75)(37.5) + (2)(50)(12)(69) \\
 &= 364.05 \times 10^3 \text{ mm}^3 = 364.05 \times 10^{-6} \text{ m}^3 \\
 t &= 100 \text{ mm} = 0.100 \text{ m}
 \end{aligned}$$



$$\tau_{\max} = \frac{VQ}{It} = \frac{(10 \times 10^3)(364.05 \times 10^{-6})}{(39.58 \times 10^{-6})(0.100)} = 920 \times 10^3 \text{ Pa}$$

$$\tau_{\max} = 920 \text{ kPa} \quad \blacktriangleleft$$

$$\begin{aligned}
 (b) \quad Q &= A_1\bar{y}_1 + 2A_2\bar{y}_2 \\
 &= (100)(40)(55) + (2)(50)(12)(69) \\
 &= 302.8 \times 10^3 \text{ mm}^3 = 302.8 \times 10^{-6} \text{ m}^3 \\
 t &= 100 \text{ mm} = 0.100 \text{ m}
 \end{aligned}$$

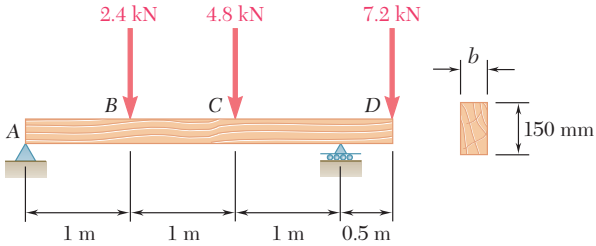


$$\tau_a = \frac{VQ}{It} = \frac{(10 \times 10^3)(302.8 \times 10^{-6})}{(39.58 \times 10^{-6})(0.100)} = 765 \times 10^3 \text{ Pa}$$

$$\tau_a = 765 \text{ kPa} \quad \blacktriangleleft$$

PROBLEM 6.18

For the beam and loading shown, determine the minimum required width b , knowing that for the grade of timber used, $\sigma_{\text{all}} = 12 \text{ MPa}$ and $\tau_{\text{all}} = 825 \text{ kPa}$.



SOLUTION

$$\begin{aligned} +\curvearrowright \Sigma M_D = 0: & -3A + (2)(2.4) + (1)(4.8) - (0.5)(7.2) = 0 \\ & A = 2 \text{ kN } \uparrow \end{aligned}$$

Draw the shear and bending moment diagrams.

$$|V|_{\text{max}} = 7.2 \text{ kN} = 7.2 \times 10^3 \text{ N}$$

$$|M|_{\text{max}} = 3.6 \text{ kN} \cdot \text{m} = 3.6 \times 10^3 \text{ N} \cdot \text{m}$$

Bending:

$$\begin{aligned} \sigma &= \frac{M}{S} \\ S_{\text{min}} &= \frac{|M|_{\text{max}}}{\sigma} \\ &= \frac{3.6 \times 10^3}{12 \times 10^6} \\ &= 300 \times 10^{-6} \text{ m}^3 \\ &= 300 \times 10^3 \text{ mm}^3 \end{aligned}$$

For a rectangular section,

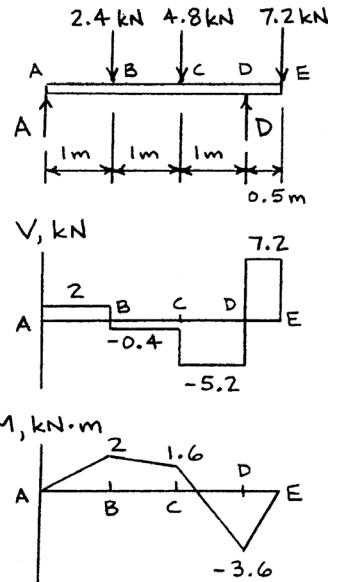
$$\begin{aligned} S &= \frac{1}{6}bh^2 \\ b &= \frac{6S}{h^2} = \frac{(6)(300 \times 10^3)}{(150)^2} = 80 \text{ mm} \end{aligned}$$

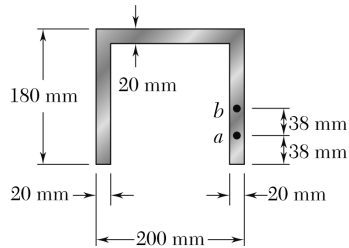
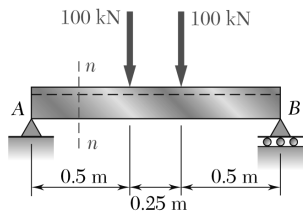
Shear: Maximum shearing stress occurs at the neutral axis of bending for a rectangular section.

$$\begin{aligned} A &= \frac{1}{2}bh, \quad \bar{y} = \frac{1}{4}h, \quad Q = A\bar{y} = \frac{1}{8}bh^2 \\ I &= \frac{1}{12}bh^3 \quad t = b \\ \tau &= \frac{VQ}{It} = \frac{V(\frac{1}{8}bh^2)}{(\frac{1}{12}bh^3)(b)} = \frac{3V}{2bh} \\ b &= \frac{3V}{2h\tau} = \frac{(3)(7.2 \times 10^3)}{(2)(150 \times 10^{-3})(825 \times 10^3)} = 87.3 \times 10^{-3} \text{ m} \end{aligned}$$

The required value of b is the larger one.

$$b = 87.3 \text{ mm} \quad \blacktriangleleft$$





PROBLEM 6.21

For the beam and loading shown, consider section $n-n$ and determine the shearing stress at (a) point a , (b) the shearing stress at point b .

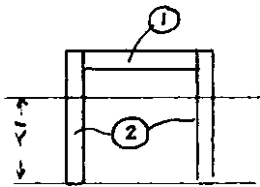
SOLUTION

$$R_A = R_B = 100 \text{ kN}$$

At section $n-n$,

$$V = 100 \text{ kN}$$

Locate centroid and calculate moment of inertia.



Part	$A(\text{mm}^2)$	$\bar{y}(\text{mm})$	$A\bar{y}(\text{mm}^3)$	$d(\text{mm})$	$Ad^2(\text{mm}^4)$	$\bar{I}(\text{mm}^4)$
①	3200	170	544000	55.4	9821312	106667
②	7200	90	648000	24.6	4357152	19440000
Σ	10400		1192000		14178464	19546667

$$\bar{Y} = \frac{\Sigma A\bar{y}}{\Sigma A} = \frac{1192000}{104.00} = 114.6 \text{ mm}$$

$$I = \Sigma Ad^2 + \Sigma \bar{I} = 14178464 + 19546667 = 33.725 \times 10^6$$

$$(a) \quad Q_a = A\bar{y} = (20)(38)(114.6 - 19) = 72656 \text{ mm}^3$$

$$t = 20 \text{ mm}$$

$$\tau_a = \frac{VQ}{It} = \frac{(100000)(72656)}{(33.725 \times 10^6)(20)} = 10.8 \text{ MPa}$$

$$(b) \quad Q_b = A\bar{y} = (20)(76)(114.6 - 38) = 1164.32 \text{ mm}^3$$

$$t = 20 \text{ mm}$$

$$\tau_b = \frac{VQ}{It} = \frac{(100000)(1164.32)}{(33.725 \times 10^6)(20)} = 17.3 \text{ MPa}$$

